## MEMORANDUM

DATE: June 26, 2020
TO: City of Ilwaco
FROM: Dana Beckwith, PE, PTOE
Richard Martin, EIT
SUBJECT: Ilwaco Comprehensive Plan Update Traffic Planning Support
P18-085-000
This memorandum summarizes the traffic impact analysis associated with the Transportation Element of the Comprehensive Plan update for the City of Ilwaco, Washington. The purpose of this analysis is to evaluate the existing transportation infrastructure throughout the City of Ilwaco, to identify potential improvements required to adequately serve future traffic conditions based on standards established by the Washington State Department of Transportation (WSDOT), the City of Ilwaco, and Pacific County.

This traffic impact analysis considers the following elements:

- Introduction
- Existing Conditions
- Safety Analysis
- Traffic Data Analysis
- Pedestrian and Bicycle Facility Evaluation
- Transportation Demand Management Strategies
- Regional and Local Agency Comprehensive Plan Review
- Mitigation Strategy and Budget Estimates
- Results and Recommendations


## INTRODUCTION

## Project Description

The City of Ilwaco is located in the southwest corner of the State of Washington, south of the City of Seaview and north of Cape Disappointment State Park. It is located along a segment of State Highway 101 that connects the city to Astoria and Raymond, among other communities along the coast. One of the main employers and trip generators in the City is the Port of Ilwaco. The Port generates land, water, and air traffic, detailed later in the memorandum.

There is one signalized intersection within the City, located at the intersection of $1^{\text {st }}$ Avenue / Spruce Street (US 101). Four study intersections were identified through coordination with City staff for capacity and crash analysis, as shown in Figure 1:

- $1^{\text {st }}$ Avenue N (US 101) / School Road
- $\quad 2^{\text {nd }}$ Avenue SW / N Head Road / Spruce Street W (US 101)
- $1^{\text {st }}$ Avenue N / Spruce Street E (US 101)
- Spruce Street E (US 101) / Elizabeth Avenue NE


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© $=$ Study Intersection
Figure 1: Vicinity Map

Limited infrastructure is present for alternative modes of transportation throughout the city. There is one bike lane in the city and intermittent pedestrian infrastructure. There are four transit stops that serve three bus lines. Further information about alternative modes is detailed in the Existing Conditions section.

The evaluation summarized in this memorandum provides information necessary to update the Transportation Element of the Ilwaco Comprehensive Plan.

## EXISTING CONDITIONS

Existing transportation conditions were evaluated in the vicinity of study intersections in Ilwaco, WA. All modes of travel were evaluated, including pedestrian, bicycles, transit and motor vehicles. The existing transportation conditions for the roadways relevant to the four study intersections are summarized in Table 1.

## Transit Infrastructure

Ilwaco hosts several transit facilities in the study area.
Pacific Transit System, headquartered in Raymond, Washington, provides bus service to Ilwaco via three bus lines:

- Line 20 - Long Beach Peninsula: Runs weekdays and Saturday between Ilwaco and Oysterville.
- Line 24 - Ilwaco/Astoria: Runs weekdays between Ilwaco and Astoria.
- Line 50 - Ilwaco to Astoria to South Bend: Runs weekdays from Ilwaco to Astoria to South Bend.

All three lines stop at the Port of Ilwaco with a transit stop located just west of Pearl Street SE.
Additionally, Line 20 currently has three bus stops along $1^{\text {st }}$ Avenue $N$ (US 101) north of downtown that include:

- The Ilwaco High School
- The Ilwaco Timberland Library
- Intersection of $1^{\text {st }}$ Avenue N at Spruce Street E


## Existing Bicycle and Pedestrian Infrastructure

The city's only dedicated bicycle infrastructure is a $1^{\text {st }}$ Avenue $S$ bike lane on the west side of the road from Eagle Street SW to Waterfront Way. Other roadways within the city require bicycles to share the roadway.

Sidewalks are generally continuous along Spruce Street E (US 101) and Lake Street SE within the downtown area of Ilwaco. Spruce Street W does not have sidewalks on the north side of the road for a small section before it turns into $N$ Head Road. Continuous sidewalks are present along $2^{\text {nd }}$ Avenue SW (though sometimes overgrown with vegetation) until the intersection at Spring Street SW. A full inventory of existing sidewalks is available in Figure 2. From a visual evaluation, the conditions of the sidewalks vary. Pedestrian ramps need to be evaluated to determine if they meet current ADA compliance and a program developed to upgrade those that don't.

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Table 1 Existing Conditions Within Study Area

| Roadway | Functional Classification ${ }^{1}$ | Posted <br> Speed <br> Limit | Sidewalks ${ }^{2}$ | Transit | Bike Lanes | Lane Geometry | On-Street Parking |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{gathered} 1^{\text {st }} \text { Ave N (US } \\ \text { 101) } \end{gathered}$ | Arterial | $\begin{gathered} 35 \\ \mathrm{mph} \end{gathered}$ | Both sides downtown | Pacific <br> Transit <br> System <br> Line 20 | None | One 9'-10' lane in each direction | Both sides |
| Spruce St E (US 101) |  | $\begin{gathered} 25 \\ \mathrm{mph} \end{gathered}$ | Both sides | None | None | One 9'-10' lane in each direction | Both sides |
| Cpt Robert Gray Dr (SR 100) |  | $\begin{gathered} 25 \\ \mathrm{mph} \end{gathered}$ | None | None | None | One $10^{\prime}$ lane in each direction | None |
| $1^{\text {st }}$ Ave S | Major Collector | $\begin{gathered} 25 \\ \mathrm{mph} \end{gathered}$ | Both sides until Jessie's Drive | None | West side from Eagle St to Waterfront Wy | One 9'-10' lane in each direction | Both sides |
| $2^{\text {nd }}$ Ave SW |  | $\begin{gathered} 25 \\ \mathrm{mph} \end{gathered}$ | West side only | None | None | One 9'-10' lane in each direction | West side |
| Brumbach Ave NE |  | $\begin{gathered} 20 \\ \mathrm{mph} \end{gathered}$ | Continuous on east side intermittent along west | None | None | One 12 ' lane <br> in each direction | Intermittent along both sides |
| N Head Rd |  | $\begin{gathered} 25 \\ \mathrm{mph} \end{gathered}$ | None | None | None | One 9'-10' lane in each direction | None |
| Howerton Wy SE |  | $\begin{gathered} 25 \\ \mathrm{mph} \end{gathered}$ | Both sides between Waterfront Wy SE and Elizabeth Ave SE | Pacific <br> Transit <br> Lines <br> 20, 24 <br> and 50 | None | One 14 ' lane <br> in each direction | North Side |
| Spruce Street <br> W |  | $\begin{gathered} 25 \\ \mathrm{mph} \end{gathered}$ | South side only | None | None | One 12' lane in each direction | Both sides |
| Elizabeth <br> Ave SE |  | $\begin{gathered} 25 \\ \mathrm{mph} \end{gathered}$ | Both sides until Port | None | None | One 9'-10' lane in each direction | None |

${ }^{1}$ Based on the Transportation Element of the 2015 City of Ilwaco Comprehensive Plan.
${ }^{2}$ Sidewalk inventories fully detailed in Figure 2.
The Discovery Trail connects Cape Disappointment to the City. The trail is paved between $2^{\text {nd }}$ Avenue SW and the intersection with $N$ Head Road to the west. Access to the trail head at $2^{\text {nd }}$ Avenue SW is limited and unimproved.


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Existing Sidewalk
No Existing Sidewalk
Multi-Use Facility

Figure 2: City of Ilwaco Sidewalk Inventory

## Air and Water Infrastructure

The Port of Ilwaco is a major shipping and fishing hub. The Port hosts mooring space for commercial and recreation vessels and provides trailer parking for launching such vessels at the end of Outer Harbor Way SE. Cape Disappointment hosts a National Motor Lifeboat School, and provides mooring for such vessels at US Coast Guard Station Cape Disappointment. The Port also owns and operates the Port of Ilwaco Airport on Stringtown Road east of downtown Ilwaco. This airport is not part of the City of Ilwaco, but provides facilities for local air travel in the region. The airport can accommodate small singleengine airplanes ${ }^{1}$.

## SAFETY ANALYSIS

## Crash Data Review

Crash data for a five-year period from January 2014 through December 2018 was obtained from the Washington State Department of Transportation. This data was reviewed to identify traffic safety concerns at the study intersection. A copy of the crash data is included in Appendix A.

The crash rate presented in Table 2 is based on the number of crashes per million entering vehicles (MEV). Typically, an intersection is considered unsafe if its crash rate exceeds the threshold of 1.0 crashes per MEV.

Table 2: Five Year Crash Rate

| Location | Crash History (Years) | Number of <br> Crashes | Crashes per year | Annual Traffic Entering (veh/yr)* | $\underset{* *}{\text { C.M.E.V. }}$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $1^{\text {st }}$ Avenue N (US 101) / School Road | 5 | 1 | 0.2 | 1,533,000 | 0.261 |
| $2{ }^{\text {nd }}$ Avenue SW / N Head Road / Spruce Street W |  | 0 | 0 | 438,000 | 0 |
| $1{ }^{\text {st }}$ Avenue N (US 101) / Spruce Street E |  | 2 | 0.4 | 1,533,000 | 0.261 |
| Spruce Street E (US 101) / Elizabeth Avenue NE |  | 1 | 0.2 | 1,022,000 | 0.196 |

Note: * From WSDOT Traffic GeoPortal
** C.M.E.V. - Crashes per million entering vehicles.
Table 2 shows that none of the study intersections exceed the 1.0 crashes per MEV safety threshold. A further evaluation of crashes by type is summarized in Table 3. No crashes were reported in 2014 or 2015, and no rear-end or single-vehicle crash types were reported. Furthermore, no crashes occurred at the intersection of $2^{\text {nd }}$ Avenue / Head Road / Spruce Street during the analysis period.

As shown in Table 3, the crash types varied and no specific reoccurring crash patterns were identified. No fatalities or severe injury crashes were reported during the study period.

[^0]Table 3: Crash Types at Study Intersections

| Crash Type | Intersection Data by Year |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| YEAR | $\mathbf{2 0 1 6}$ | $\mathbf{2 0 1 7}$ | $\mathbf{2 0 1 8}$ | Total |
| $\mathbf{1}^{\text {st }}$ Avenue N (US 101) / School Road |  |  |  |  |
| Bike-involved | $0(0)$ | $0(0)$ | $1(1)$ | $1(1)$ |
| $\mathbf{1}^{\text {st }}$ Avenue N (US 101) / Spruce Street E |  |  |  |  |
| Entering at angle | $(0)$ | $1(0)$ | $0(0)$ | $1(0)$ |
| From opposite direction | $1(0)$ | $0(0)$ | $0(0)$ | $1(0)$ |
| Spruce Street E (US 101) / Elizabeth Avenue NE |  |  |  |  |
| From same direction - all others | $0(0)$ | $1(0)$ | $0(0)$ | $1(0)$ |
| Total | $1(0)$ | $2(0)$ | $1(1)$ | $\mathbf{4 ( 1 )}$ |

Note: $\mathrm{X}(\mathrm{X})=$ Total Crashes (Injury Crashes)

## TRAFFIC DATA ANALYSIS

## 2019 Existing Conditions

An intersection performance analysis was conducted to document the existing operations for the study intersections and to develop a baseline for analyzing future intersection operational needs associated with the proposed development. Intersection operations were analyzed for the current peak hour performance. The PM peak period (4:00 to 6:00 PM) was analyzed for this evaluation. Traffic count data was gathered on Wednesday, October 16, 2019. Existing traffic volumes and lane configurations are shown in Figure 3. The detailed turn movement count data has been included in Appendix B.

The level of service analyses presented in this memorandum has been completed using the Synchro (Version 10) analysis software. Synchro is based on the Highway Capacity Manual (HCM) $6^{\text {th }}$ Edition methodology. The study intersections analyzed includes the following local intersections:

- $\quad 1^{\text {st }}$ Avenue N (US 101) / School Road
- $\quad 2^{\text {nd }}$ Avenue SW / N Head Road / Spruce Street W (US 101)
- $\quad 1^{\text {st }}$ Avenue N / Spruce Street E (US 101)
- Spruce Street E (US 101) / Elizabeth Avenue NE

The City of Ilwaco utilizes level of service standards to identify the maximum levels of congestion acceptable to the community and the threshold to determine transportation system deficiencies and improvement needs. The City utilizes the following level of service standards ${ }^{2}$ :

- Arterial Streets: Level of Service D or better
- Collector Streets: Level of Service C or better

WSDOT also utilizes level of service standards and has jurisdiction over four state highways in the vicinity of Ilwaco: SR 100, US 101 and Alternate US 101. All four have a State-designated ${ }^{3}$ minimum level

[^1]
of service standard of LOS C or better. As part of the Comprehensive Plan update, it is recommended that the City of Ilwaco adopt a LOS standard for State Highways of LOS C or better.

The following Table 4 summarizes the existing traffic operations for the study intersections. For signalized intersections, the average total delay is reported. For unsignalized intersections, the highest control delay is reported. The detailed analysis results have been included in Appendix C .

Table 4: Existing Intersection Performance Summary

| Intersection | Year 2019 <br> Level of <br> Service |  |  |
| :---: | :---: | :---: | :---: |
|  | Control <br> Delay (Sec) | V/C |  |
| $1^{\text {st }}$ Avenue N (US 101) / School Road | 10.9 | B | 0.11 |
| $2^{\text {nd }}$ Avenue SW / N Head Road / Spruce Street W (US 101) | 9.7 | A | 0.04 |
| $1^{\text {st }}$ Avenue N / Spruce Street E (US 101) | 5.4 | A | 0.18 |
| Spruce Street E (US 101) / Elizabeth Avenue NE | 10.3 | B | 0.03 |

Under 2019 existing conditions, all study intersections operate at an acceptable level of service.

## 2040 Horizon Year Conditions

A Future Conditions Analysis was conducted to determine the expected traffic operating conditions for the study intersections for the 2040 horizon year assigned by the City.

## Background Growth

A background growth rate of 1.1 percent per year was used to project future conditions, consistent with the Land Use element of the Ilwaco Comprehensive Plan. ${ }^{4}$ The growth rate was determined by studying population and employment growth projections. The population projections are provided in the Land Use Element of the Comprehensive Plan and were developed in coordination with Pacific County and The Washington State Office of Financial Management. The employment projections were provided by the Washington Employment Security Department. Population growth was projected to be 0.75 percent per year, while employment growth was projected to be 1.1 percent per year. Based on the available information, the employment growth was used in developing the future traffic volumes. This rate was considered the more conservative without over-counting the forecasted volumes.

## 2040 Horizon Year Performance

The following Table 5 summarizes the traffic operations for the study intersections. Under 2040 horizon year conditions, all study intersections operate at an acceptable level of service. A queueing analysis was performed, and no queueing penalties were found in the system. The average and $95^{\text {th }}$ percentile queues for the signalized intersection of $1^{\text {st }}$ Avenue (US 101) / Spruce Street is found in Table 6. Based on the capacity and queueing analyses, no turn lane or signal warrant analysis is necessary. Figure 4 shows the future volumes.

[^2]

Table 5: 2040 Horizon Year Intersection Performance Summary

| Intersection | Year 2040 |  |  |
| :---: | :---: | :---: | :---: |
|  | Control Delay $(\mathrm{Sec})^{\mathrm{a}, \mathrm{b}}$ | Level of Service | V/C |
| $1{ }^{\text {st }}$ Avenue N (US 101) / School Road | 11.7 | B | 0.14 |
| $2^{\text {nd }}$ Avenue SW / N Head Road / Spruce Street W (US 101) | 10.0 | B | 0.07 |
| $1{ }^{\text {st }}$ Avenue $\mathrm{N} /$ Spruce Street E (US 101) | 5.6 | A | 0.22 |
| Spruce Street E (US 101) / Elizabeth Avenue NE | 10.7 | B | 0.03 |

This capacity analysis reports the projected travel demand during a typical weekday during the Fall of the year when school is in session and fishing season is at its peak. Under these conditions, no capacityrelated issues were observed to be present at the study intersection. It is acknowledged that these conditions are different during peak weekend periods during the summer, when tourism peaks for residential vacation rentals and Cape Disappointment State Park. The detailed analysis results have been included in Appendix D.

## PEDESTRIAN AND BICYCLE FACILITY EVALUATION

The primary pedestrian and bicycle routes considered in this evaluation deal with serving such traffic between downtown Ilwaco and the Port. Elizabeth Avenue SE and 1st Avenue S are the primary roadways connecting the two areas of the city for motor vehicle traffic. Sidewalk infill has occurred at Elizabeth Avenue SE between Spruce Street E and Howerton Avenue since the last Comprehensive Plan update. Sidewalk infill for roadways connecting downtown to the Port is still needed along 1st Avenue S, Pearl Avenue SE, and Advent Avenue SE. Many of the existing pedestrian ramps need to be evaluated for meeting current ADA standards. Existing pedestrian infrastructure is detailed in Figure 2.

Bicycle infrastructure should be considered for ways to serve the community on Ilwaco. These considerations should include connecting gathering places, places of employment and places of interest. At a minimum, these should include schools, the Port, the hospital, parks and Downtown. Roads most supportive of bike lanes and/or sharrows include School Road, Lake Street SE, Brumbach Avenue NE, Advent Avenue, 1st Avenue S, Spruce Street, Howerton Avenue, and Elizabeth Avenue. The specific recommendations for bike lanes vs sharrows are provided in the TDM analysis later in the memorandum. Furthermore, consideration should be made for direction connections to the Discovery Trail from Downtown and the Port. Existing and proposed bicycle infrastructure is detailed in Figure 5.


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Existing Bike Lane
Proposed Bike Lanes
Proposed Bike Sharrows

Figure 5: City of Ilwaco Bicycle Infrastructure

## FUTURE TRANSPORTATION SYSTEM NEEDS

The Transportation Element of the Comprehensive Plan lists the following concerns that may impact the City of Ilwaco transportation system:

1) Encourage heavy commercial and industrial truck traffic on US 101 to use the Elizabeth Avenue SE - Howerton Avenue - 1st Avenue S couplet to access the Port of Ilwaco.
2) Promote the use of the Alternate US 101 to relieve traffic congestion in Ilwaco during peak traffic periods.
3) Reduce traffic congestion at Spruce Street E and 1st Avenue N by adding turn lanes.
4) Investigate safe bicycle routes that connect major interest points in the city to ensure rider safety.
5) Improve pedestrian circulation and safety throughout the city, with an emphasis on providing sidewalks between downtown and the Port of Ilwaco.
6) Work with Pacific Transit to increase service to Ilwaco from Long Beach and other destinations within the county.

Regarding each of the listed concerns, the following considerations were developed:

1) To encourage heavy vehicles to use the Elizabeth Avenue SE - Howerton Avenue - 1st Avenue S couplet to access the Port of Ilwaco, improve advance signing to provide direction into the Port area. In addition, Port businesses to promote the desired route for freight movements into and out of the Port area.
2) Improve the existing signing at the intersection of US 101 at Alternate US 101. Redirect Long Beach traffic along Alternate US 101. This would identify Ilwaco as the only destination along the US 101 main route. Change the designation of the main and alternate routes to emphasize the route that bypasses Ilwaco. Similarly, provide signing for southbound traffic north of the 40th Street / US 101 intersection to direct them to Alternate US 101. Signage that details this fact would encourage these vehicles to do so.
3) The capacity analysis showed little delay during the typical weekday PM peak period during fishing season. If turn lanes are to be added to the intersection of Spruce Street E at 1st Avenue $N$, justification must come from peak tourism season or from outside of the capacity and queueing analyses. As congestion increases, the first turn lanes to be investigated are southbound left-turn and westbound right-turn lanes.
4) Develop the City's bike network:
a. A simple approach is to provide shared lane markings (sharrows) for those roadways were bikes are anticipated to travel the most. Sharrows should be considered along School Road, Advent Avenue NE and SE, Elizabeth Avenue NE and SE, Spruce Street E and W, Lake Street SE and SW, Pearl Avenue SE, Howerton Way, $1^{\text {st }}$ Avenue S, and Main Street SW and SE.
b. Develop a plan that considers bike lanes on roadways with sufficient pavement width and right-of-way to accommodate those lanes. Consider pavement improvements and bike lanes along 2nd Avenue SW, Lake Street SW and SE, Eagle Street SW, Spruce Street W, and Spruce Street E (US 101). This will encourage bike traffic between Discover Trail, Downtown Ilwaco, the Port and other points of interest. A map of existing and potential bicycle facilities is presented in Figure 5.
5) Continue to develop the City's pedestrian network:
a. Provide paving and sidewalk infill along both sides of $1^{\text {st }}$ Avenue $S$, Advent Avenue SE , Quaker Avenue SE, and Pearl Avenue SE to connect downtown and the Port of Ilwaco.
b. Develop a plan to bring existing sidewalks and pedestrian facilities up to current ADA.
c. The lot of land bounded by $1^{\text {st }}$ Avenue $S$ and Jessie's Drive - Taxlot Number 73048003014 - is a gravel lot offering no infrastructure for pedestrians, bicycles, or other modes of transportation. While the intersection to the north provides a crosswalk for pedestrians to access sidewalks, creating sidewalks on the east side of $1^{\text {st }}$ Avenue S and the north side of Howerton Avenue to Waterfront Way would provide better access for pedestrians and incentivize them to walk between Downtown and the Port.
6) Pacific Transit Services monitors citizen feedback on transit services. Continue to coordinate with Pacific Transit on ridership needs and system expansion opportunities when warranted by demand.
7) Require short-term vacation rentals such as AirBnB or VRBO, hotels, inns, RV parks and motels to provide options for alternative modes of transportation. This can include pamphlets detailing the bicycle and pedestrian options in and around the City, transit passes for local bus services, or bicycles to ride around the City.

## CAPITAL IMPROVEMENT NEEDS AND BUDGET ESTIMATES

The Transportation Element of the most recent Ilwaco Comprehensive Plan lists roadway capital improvements to take place between 2015 and 2021. This table is reproduced as Table 6 with the end date extended to 2026. Additional projects have been added to the end of the table as projected future improvements. The table includes budget estimates and elements necessary to determine mitigation strategy.

Table 6: 2015-2020 Transportation Improvement Plan

| Priority / <br> Year | Street | Project Description | Funding <br> Class |  |
| :--- | :--- | :--- | :--- | :--- | :--- |
| $1 / 2016$ | Adelia \& Spruce Streets | Reconstruction and paving from US 101 cut-off to Lake Street, including <br> parking area | Local | Cost |

## REGIONAL AND LOCAL AGENCY COMPREHENSIVE PLAN REVIEW

This review was conducted to determine if the goals of the Transportation Element of the City of Ilwaco's Comprehensive Plan align with those of the region and surrounding agencies.

The Cowlitz-Wahkiakum Council of Governments developed a Regional Transportation Plan (RTP) for the Southwest Washington Regional Transportation Planning Organization. This plan was developed to coincide with the vision and policy of the Washington and Oregon Transportation Plans. The plan includes the transportation system goals and policies for the counties surrounding the City of Ilwaco including: Grays Harbor, Lewis, Cowlitz, Wahkiakum and Pacific Counties. A review of the guiding principles, goals, and policies of the RTP yielded the following items:

## Washington State Transportation System Plan Policy Goals

- PRESERVATION: To maintain, preserve, and extend the life and utility of prior investments in transportation systems and services.
- SAFETY: To provide for and improve the safety and security of transportation customers and the transportation system.
- MOBILITY: To improve the predictable movement of goods and people throughout Washington state.
- ENVIRONMENT: To enhance Washington's quality of life through transportation investments that promote energy conservation, enhance healthy communities, and protect the environment.
- STEWARDSHIP: To continuously improve the quality, effectiveness, and efficiency of the transportation system.
- ECONOMIC VITALITY: To promote and develop transportation systems that stimulate, support, and enhance the movement of people and goods to ensure a prosperous economy.


## Oregon Transportation Plan Policy Goals

- MOBILITY AND ACCESSIBILITY: Provide a balanced, efficient, and integrated transportation system that ensures interconnected access to all areas of the state, the nation, and the world. Promote transportation choices that are reliable, accessible, and cost-effective.
- MANAGEMENT OF THE SYSTEM: Improve the efficiency of the transportation system by optimizing operations and management. Manage transportation assets to extend their life and reduce maintenance costs.
- ECONOMIC VITALITY: Expand and diversify Oregon's economy by transporting people, goods, services, and information in safe, energy efficient, and environmentally sound ways. Provide Oregon with a competitive advantage by promoting an integrated freight system.
- SUSTAINABILITY: Meet present needs without compromising the ability of future generations to meet their needs from the joint perspective of the environment, economy, and communities. Encourage conservation and communities that integrate land use and transportation choices.
- SAFETY AND SECURITY: Build, operate, and maintain the transportation system so that it is safe and secure. Take into account the needs of all users: operators, passengers, pedestrians, and property owners.
- FUNDING THE TRANSPORTATION SYSTEM: Create sources of revenue that will support a viable transportation system today and in the future. Expand ways to fund the system that are fair and fiscally responsible.
- COORDINATION, COMMUNICATION, AND COOPERATION: Foster coordination, communication, and cooperation between transportation users and providers so various means of transportation function as an integrated system. Work to help all parties align interests, remove barriers, and offer innovative, equitable solutions.


## SW Washington Regional Transportation Planning Organization Policy Element

The Southwest Washington Regional Transportation Plan consists of the following policy elements:

- Guiding Principles
- Preserve and improve the regional transportation system through partnerships in order to maximize investment.
- Provide an integrated transportation system that encourages the use of all modes by offering accessible, safe, and efficient travel options.
- Encourage the preservation and enhancement of public transportation programs and infrastructure throughout the Southwest Washington RTPO and explore opportunities for expanding service to increase access to jobs, services, and other key destinations.
- Support the region's economic vitality through ensuring the transportation network addresses inter- and intra-regional accessibility and mobility needs for both people and goods.
- Goals
- Goal 1: Promote and support a transportation system that strengthens the region's economic competitiveness.
- Goal 2: Preserve and enhance the region's existing transportation infrastructure and facilities.
- Goal 3: Develop an integrated non-motorized transportation system.
- Goal 4: Maintain, modernize, and enhance a sustainable and comprehensive public transportation system.
- Goal 5: Maintain and enhance a regional transportation system that is safe and accessible for multiple travel modes.

The following considerations were developed regarding the goals set forth in the RTP.

- Goal 1: Promote and support a transportation system that strengthens the region's economic competitiveness.
- TDM measures are recommended to improve tourism, one of the main economic drivers in the region that utilize ground transportation. Improved access to the Port, Cape Disappointment and other points of community interest for all travel modes will encourage greater visitation to Ilwaco.
- Goal 2: Preserve and enhance the region's existing transportation infrastructure and facilities.
- Capital improvements and traffic capacity have been analyzed. Improvements have been identified to enhance the existing transportation network for all travel modes and preserve the existing infrastructure.
- Goal 3: Develop an integrated non-motorized transportation system.
- Existing non-motorized transportation modes have been identified and improvements to the transportation system are recommended with regards to bicycle, pedestrian, and transit facilities in and around the City of Ilwaco.
- Goal 4: Maintain, modernize, and enhance a sustainable and comprehensive public transportation system.
- Continued coordination with Pacific Transit System and neighboring communities is encouraged to maintain and promote public transportation services in underserved areas and expand the system to support increases in ridership.
- Goal 5: Maintain and enhance a regional transportation system that is safe and accessible for multiple travel modes.
- Bike facilities, a more complete sidewalk system, improved ADA-accessible pedestrian ramps and improvements to routing of freight along US 101 and Alternate Route 101 are recommended to maintain and enhance safe and accessible travel.

The goals of the Transportation Element of the updated Ilwaco Comprehensive Plan align with those set forth in the regional RTP.

## RESULTS AND RECOMMENDATIONS

An evaluation of the City of Ilwaco's existing transportation system was conducted to support the update to the City's Transportation Element of the Comprehensive Plan. Study intersection were identified through coordination with City staff.

A crash analysis was conducted for the years 2014 to 2018 to identify any safety deficiencies. The crash analysis did not identify any reoccurring crash patterns or crash rates that would identify an ongoing safety issues at the study intersections.

Capacity and queueing analyses were conducted to determine existing and future year 2040 capacity operations at four study intersections. Based on these analyses, no capacity deficiencies were identified, and no mitigations are recommended at the study intersections.

A review of the existing pedestrian and bicycle infrastructure in the city identified areas of improvement for both modes of transportation. Additions and improvements to infrastructure for these modes were identified to encourage active transportation modes. The need to identify and upgrade pedestrian facilities have also been identified to bring existing infrastructure up to current ADA standards.

A review of regional and local agency plans to determine the compliance of the proposed update of the Transportation Element of the Ilwaco Comprehensive Plan was performed. The goals of the planned update were found to align with those of the region.

## Appendix

## Appendix A: Crash Data

OFFICER REPORTED CRASHES THAT OCCURRED AT THE FOLLOWING INTERSECTIONS IN THE CITY OF ILWACO

## SR 100 (aka 2nd Ave, MP 4.66-4.68) @ HEAD RD / SPRUCE ST - No Reported Crashes

SR 101 (aka Spruce St, MP 11.18-11.22) @ ELIZABETH AVE
SR 101 (aka Spruce St, MP 11.55-11.59) @ 1st AVE
SR 101 (aka 1st Ave, MP 11.81-11.85) @ SCHOOL RD

## 01/01/2014-12/31/2018

Under 23 U.S. Code $\$ 148$ and 23 U.S. Code § 409, safety data, reports, surveys, schedules, lists compiled or collected for the purpose of
identifying, evaluating, or planning the safety enhancement of potential crash sites, hazardous roadway conditions, or railway-highway crossing are not subject to discovery or admitted into evidence in a Federal or State court proceeding or considered for other purposes in any action for damages arising from any occurrence at a location mentioned or addressed in such reports, surveys, schedules, lists, or data.

| JURISDICTION | COUNTY | CITY | PRIMARY TRAFFICWAY | MILEPOST |  | $\begin{array}{\|c\|} \hline \text { SR ONLY } \\ \text { HISTORY / } \\ \text { SUSPENSE } \\ \text { IND } \end{array}$ | REPORT NUMBER | DATE | TIME | MOST SEVERE INJURY TYPE | \# | \# |  | $\begin{array}{c\|l} \# & \# \\ \# & \mathrm{~B} \\ \mathrm{P} & \mathrm{I} \\ \mathrm{E} & \mathrm{~K} \\ \mathrm{D} & \mathrm{E} \\ \mathrm{~S} & \mathrm{~S} \end{array}$ | VEHICLE 1 TYPE | VEHICLE 2 TYPE | JUNCTION RELATIONSHIP | WEATHER | $\begin{array}{\|c\|} \text { ROADWAY } \\ \text { SURFACE } \\ \text { CONDITION } \end{array}$ | LIGHTING CONDITION |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| State Route | Pacific | Ilwaco | 101 | 11.19 |  | No | 3625548 | 06/05/2017 | 09:18 | No Apparent Injury | 0 | 0 | 3 | 00 | Pickup,Panel Truck or Vanette under 10,000 lb | Passenger Car | Intersection Related but Not at Intersection | Clear or Partly Cloudy | Dry | Daylight |
| State Route | Pacific | Ilwaco | 101 | 11.57 |  | No | 3612124 | 03/17/2016 | 17:34 | No Apparent Injury | 0 | 0 | 2 | 00 | Pickup,Panel Truck or Vanette under 10,000 lb | Passenger Car | At Intersection and Related | Clear or Partly Cloudy | Dry | Daylight |
| State Route | Pacific | Ilwaco | 101 | 11.59 |  | No | 3625573 | 07/12/2017 | 13:00 | No Apparent Injury | 0 | 0 | 2 | 00 | Passenger Car | Pickup,Panel Truck or Vanette under 10,000 lb | At Driveway within Major Intersection | Clear or Partly Cloudy | Dry | Daylight |
| State Route | Pacific | Ilwaco | 101 | 11.83 |  | No | 3757943 | 12/07/2018 | 11:46 | Possible Injury | 1 | 0 | 1 | 01 |  | Truck <br> (Flatbad,Van,etc) | At Intersection and Related | Overcast | Dry | Daylight |


| FIRST COLLISION TYPE / OBJECT STRUCK | VEHICLE 1 <br> ACTION | VEHICLE 2 ACTION |  |  | VEHICLE 2 <br> COMPASS <br> DIRECTION <br> FROM | VEHICLE 2 <br> COMPASS DIRECTION TO | MV DRIVER CONTRIBUTING CIRCUMSTANCE 1 (UNIT 1) | $\qquad$ | $\qquad$ | BICYCLIST CONTRIBUTING CIRCUMSTANCE 1 (UNIT 1) | FIRST IMPACT LOCATION (City, <br> County \& Misc Trafficways - 2010 forward) | WA STATE <br> PLANE SOUTH - X 2010FORWAR D | WA STATE <br> PLANE <br> SOUTH-Y <br> 2010- <br> FORWAR <br> D |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| One parked--one moving | Going <br> Straight <br> Ahead | Legally Parked, Unoccupied | West | East | Vehicle <br> Backing | Vehicle Stopped | Exceeding Reas. Safe Speed | Inattention |  |  | Right Shoulder Increasing Milepost | 747226.82 | 375753.1 |
| From opposite <br> direction - one <br> left turn - one | Making Left <br> Turn | Going Straight Ahead | East | North | West | East | Improper Turn | Inattention | None |  | Lane 1 Increasing Milepost | 745281.02 | 375844.09 |
| Entering at angle | Going <br> Straight <br> Ahead | Stopped for Traffic | North | South | West | South | Inattention |  | Inattention |  | Lane 1 Decreasing Milepost | 745284.67 | 375936.66 |
| Vehicle Strikes Pedalcyclist |  | $\begin{aligned} & \hline \text { Making Right } \\ & \text { Turn } \end{aligned}$ |  |  | East | North |  |  | Other | On Wrong Side Of Road | Intersecting Road Increasing Milepost | 745407.11 | 377260.29 |

## Appendix B: Traffic Count Data



Peak-Hour: 4:20 PM -- 5:20 PM
Peak 15-Min: 4:55 PM -- 5:10 PM DATE: Wed, Oct 162019


Comments:


Peak-Hour: 4:20 PM -- 5:20 PM
Peak 15-Min: 4:25 PM -- 4:40 PM




## Appendix C: Existing HCM Reports

| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.7 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Yr |  | $\uparrow$ |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 7 | 55 | 190 | 17 | 76 | 125 |
| Future Vol, veh/h | 7 | 55 | 190 | 17 | 76 | 125 |
| Conflicting Peds, \#/hr | 18 | 18 | 0 | 18 | 18 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 86 | 86 | 86 | 86 | 86 | 86 |
| Heavy Vehicles, \% | 0 | 2 | 2 | 0 | 4 | 4 |
| Mvmt Flow | 8 | 64 | 221 | 20 | 88 | 145 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 5.9 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | 4 |  |  | \& |  |  | \$ |  |  | * |  |
| Traffic Vol, veh/h | 0 | 22 | 1 | 16 | 11 | 7 | 1 | 6 | 36 | 13 | 8 | 1 |
| Future Vol, veh/h | 0 | 22 | 1 | 16 | 11 | 7 | 1 | 6 | 36 | 13 | 8 | 1 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 1 | 1 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 73 | 73 | 73 | 73 | 73 | 73 | 73 | 73 | 73 | 73 | 73 | 73 |
| Heavy Vehicles, \% | 0 | 9 | 0 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Mvmt Flow | 0 | 30 | 1 | 22 | 15 | 10 | 1 | 8 | 49 | 18 | 11 | 1 |



|  | 4 | $\rightarrow$ | 7 | 7 |  | 4 | 4 | 4 | \% | $0$ | 1 | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \& |  |  | \$ |  |  | * |  |  | \& |  |
| Traffic Volume (veh/h) | 25 | 38 | 3 | 4 | 17 | 60 | 13 | 84 | 4 | 53 | 53 | 24 |
| Future Volume (veh/h) | 25 | 38 | 3 | 4 | 17 | 60 | 13 | 84 | 4 | 53 | 53 | 24 |
| Initial $Q(Q b)$, veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 0.99 |  | 0.97 | 0.99 |  | 0.99 | 0.99 |  | 0.96 | 0.99 |  | 0.99 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach |  | No |  |  | No |  |  | No |  |  | No |  |
| Adj Sat Flow, veh/h/ln | 1856 | 1856 | 1856 | 1811 | 1811 | 1811 | 1841 | 1841 | 1841 | 1841 | 1841 | 1841 |
| Adj Flow Rate, veh/h | 31 | 47 | 4 | 5 | 21 | 74 | 16 | 104 | 5 | 65 | 65 | 30 |
| Peak Hour Factor | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 |
| Percent Heavy Veh, \% | 3 | 3 | 3 | 6 | 6 | 6 | 4 | 4 | 4 | 4 | 4 | 4 |
| Cap, veh/h | 374 | 187 | 14 | 223 | 63 | 209 | 265 | 498 | 22 | 419 | 269 | 93 |
| Arrive On Green | 0.18 | 0.18 | 0.18 | 0.18 | 0.18 | 0.18 | 0.31 | 0.31 | 0.31 | 0.31 | 0.31 | 0.31 |
| Sat Flow, veh/h | 526 | 1044 | 81 | 55 | 354 | 1164 | 114 | 1587 | 71 | 427 | 857 | 296 |
| Grp Volume(v), veh/h | 82 | 0 | 0 | 100 | 0 | 0 | 125 | 0 | 0 | 160 | 0 | 0 |
| Grp Sat Flow(s),veh/h/ln | 1651 | 0 | 0 | 1573 | 0 | 0 | 1773 | 0 | 0 | 1580 | 0 | 0 |
| Q Serve(g_s), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Cycle Q Clear(g_c), s | 0.7 | 0.0 | 0.0 | 1.0 | 0.0 | 0.0 | 0.9 | 0.0 | 0.0 | 1.2 | 0.0 | 0.0 |
| Prop In Lane | 0.38 |  | 0.05 | 0.05 |  | 0.74 | 0.13 |  | 0.04 | 0.41 |  | 0.19 |
| Lane Grp Cap(c), veh/h | 576 | 0 | 0 | 495 | 0 | 0 | 785 | 0 | 0 | 781 | 0 | 0 |
| V/C Ratio(X) | 0.14 | 0.00 | 0.00 | 0.20 | 0.00 | 0.00 | 0.16 | 0.00 | 0.00 | 0.20 | 0.00 | 0.00 |
| Avail Cap(c_a), veh/h | 1887 | 0 | 0 | 1800 | 0 | 0 | 1992 | 0 | 0 | 1828 | 0 | 0 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(l) | 1.00 | 0.00 | 0.00 | 1.00 | 0.00 | 0.00 | 1.00 | 0.00 | 0.00 | 1.00 | 0.00 | 0.00 |
| Uniform Delay (d), s/veh | 6.3 | 0.0 | 0.0 | 6.4 | 0.0 | 0.0 | 4.5 | 0.0 | 0.0 | 4.6 | 0.0 | 0.0 |
| Incr Delay (d2), s/veh | 0.1 | 0.0 | 0.0 | 0.2 | 0.0 | 0.0 | 0.1 | 0.0 | 0.0 | 0.1 | 0.0 | 0.0 |
| Initial Q Delay(d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \%ile BackOfQ( $50 \%$ ),veh/ln | 0.1 | 0.0 | 0.0 | 0.1 | 0.0 | 0.0 | 0.1 | 0.0 | 0.0 | 0.1 | 0.0 | 0.0 |
| Unsig. Movement Delay, s/veh |  |  |  |  |  |  |  |  |  |  |  |  |
| LnGrp Delay(d),s/veh | 6.4 | 0.0 | 0.0 | 6.6 | 0.0 | 0.0 | 4.6 | 0.0 | 0.0 | 4.7 | 0.0 | 0.0 |
| LnGrp LOS | A | A | A | A | A | A | A | A | A | A | A | A |
| Approach Vol, veh/h |  | 82 |  |  | 100 |  |  | 125 |  |  | 160 |  |
| Approach Delay, s/veh |  | 6.4 |  |  | 6.6 |  |  | 4.6 |  |  | 4.7 |  |
| Approach LOS |  | A |  |  | A |  |  | A |  |  | A |  |
| Timer - Assigned Phs |  | 2 |  | 4 |  | 6 |  | 8 |  |  |  |  |
| Phs Duration (G+Y+Rc), s |  | 10.1 |  | 7.7 |  | 10.1 |  | 7.7 |  |  |  |  |
| Change Period (Y+Rc), s |  | 4.5 |  | 4.5 |  | 4.5 |  | 4.5 |  |  |  |  |
| Max Green Setting (Gmax), s |  | 18.0 |  | 18.0 |  | 18.0 |  | 18.0 |  |  |  |  |
| Max Q Clear Time (g_c+11), s |  | 2.9 |  | 2.7 |  | 3.2 |  | 3.0 |  |  |  |  |
| Green Ext Time (p_c), s |  | 0.5 |  | 0.3 |  | 0.7 |  | 0.4 |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| HCM 6th Ctrl Delay |  |  | 5.4 |  |  |  |  |  |  |  |  |  |
| HCM 6th LOS |  |  | A |  |  |  |  |  |  |  |  |  |


|  | 4 | $\rightarrow$ | $\geqslant$ | 7 |  | 4 | 4 | 4 | P |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | ¢ |  |  | ¢ |  |  | ¢ |  |  | ¢ |  |
| Traffic Volume (veh/h) | 25 | 38 | 3 | 4 | 17 | 60 | 13 | 84 | 4 | 53 | 53 | 24 |
| Future Volume (veh/h) | 25 | 38 | 3 | 4 | 17 | 60 | 13 | 84 | 4 | 53 | 53 | 24 |
| Number | 7 | 4 | 14 | 3 | 8 | 18 | 5 | 2 | 12 | 1 | 6 | 16 |
| Initial Q, veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj (A_pbT) | 0.99 |  | 0.97 | 0.99 |  | 0.99 | 0.99 |  | 0.96 | 0.99 |  | 0.99 |
| Parking Bus Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach |  | No |  |  | No |  |  | No |  |  | No |  |


| Lanes Open During Work Zone |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Adj Sat Flow, veh/h/ln | 1856 | 1856 | 1856 | 1811 | 1811 | 1811 | 1841 | 1841 | 1841 | 1841 | 1841 | 1841 |
| Adj Flow Rate, veh/h | 31 | 47 | 4 | 5 | 21 | 74 | 16 | 104 | 5 | 65 | 65 | 30 |
| Peak Hour Factor | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 |
| Percent Heavy Veh, \% | 3 | 3 | 3 | 6 | 6 | 6 | 4 | 4 | 4 | 4 | 4 | 4 |
| Opposing Right Turn Influence | Yes |  |  | Yes |  |  | Yes |  |  | Yes |  |  |
| Cap, veh/h | 374 | 187 | 14 | 223 | 63 | 209 | 265 | 498 | 22 | 419 | 269 | 93 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Prop Arrive On Green | 0.18 | 0.18 | 0.18 | 0.18 | 0.18 | 0.18 | 0.31 | 0.31 | 0.31 | 0.31 | 0.31 | 0.31 |
| Unsig. Movement Delay |  |  |  |  |  |  |  |  |  |  |  |  |
| Ln Grp Delay, s/veh | 6.4 | 0.0 | 0.0 | 6.6 | 0.0 | 0.0 | 4.6 | 0.0 | 0.0 | 4.7 | 0.0 | 0.0 |
| LL Grp LOS | A | A | A | A | A | A | A | A | A | A | A | A |
| Approach Vol, veh/h |  | 82 |  |  | 100 |  |  | 125 |  |  | 160 |  |
| Approach Delay, slveh |  | 6.4 |  |  | 6.6 |  |  | 4.6 |  |  | 4.7 |  |
| Approach LOS |  | A |  |  | A |  |  | A |  |  | A |  |


| Timer: | 1 | 2 | 3 | 4 | 5 | 6 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |


| Through Movement Data |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | :---: |
| Assigned Mvmt | 2 | 4 | 6 | 8 |  |
| Mvmt Sat Flow, veh/h | 1587 | 1044 | 857 | 354 |  |
| Right-Turn Movement Data |  |  |  |  |  |
| Assigned Mvmt | 12 | 14 | 16 | 18 |  |
| Mvmt Sat Flow, veh/h | 71 | 81 | 296 | 1164 |  |


| Left Lane Group Data |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Assigned Mvmt | 0 | 5 | 0 | 7 | 0 | 1 | 0 | 3 |
| Lane Assignment |  | $L+T+R$ |  | $L+T+R$ |  | $L+T+R$ |  | $L+T+R$ |

HCM 6th Signalized Intersection Capacity Analysis
3: 1st Avenue S/1st Avenue N \& Spruce Street W/Spruce Street E

| Lanes in Grp | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Grp Vol (v), veh/h | 0 | 125 | 0 | 82 | 0 | 160 | 0 | 100 |
| Grp Sat Flow (s), veh/h/ln | 0 | 1773 | 0 | 1651 | 0 | 1580 | 0 | 1573 |
| Q Serve Time ( g _s), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Cycle Q Clear Time (g_c), s | 0.0 | 0.9 | 0.0 | 0.7 | 0.0 | 1.2 | 0.0 | 1.0 |
| Perm LT Sat Flow (s_l), veh/h/ln | 0 | 1307 | 0 | 1313 | 0 | 1290 | 0 | 1366 |
| Shared LT Sat Flow (s_sh), veh/h/ln | 0 | 1826 | 0 | 1817 | 0 | 1795 | 0 | 1806 |
| Perm LT Eff Green (g_p), s | 0.0 | 5.6 | 0.0 | 3.2 | 0.0 | 5.6 | 0.0 | 3.2 |
| Perm LT Serve Time (g_u), s | 0.0 | 4.4 | 0.0 | 2.2 | 0.0 | 4.7 | 0.0 | 2.5 |
| Perm LT Q Serve Time (g_ps), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Time to First Blk (g_f), s | 0.0 | 2.7 | 0.0 | 0.4 | 0.0 | 1.7 | 0.0 | 0.9 |
| Serve Time pre Blk (g_fs), s | 0.0 | 0.9 | 0.0 | 0.4 | 0.0 | 1.2 | 0.0 | 0.9 |
| Prop LT Inside Lane (P_L) | 0.00 | 0.13 | 0.00 | 0.38 | 0.00 | 0.41 | 0.00 | 0.05 |
| Lane Grp Cap (c), veh/h | 0 | 785 | 0 | 576 | 0 | 781 | 0 | 495 |
| V/C Ratio (X) | 0.00 | 0.16 | 0.00 | 0.14 | 0.00 | 0.20 | 0.00 | 0.20 |
| Avail Cap (c_a), veh/h | 0 | 1992 | 0 | 1887 | 0 | 1828 | 0 | 1800 |
| Upstream Filter (I) | 0.00 | 1.00 | 0.00 | 1.00 | 0.00 | 1.00 | 0.00 | 1.00 |
| Uniform Delay (d1), s/veh | 0.0 | 4.5 | 0.0 | 6.3 | 0.0 | 4.6 | 0.0 | 6.4 |
| Incr Delay (d2), s/veh | 0.0 | 0.1 | 0.0 | 0.1 | 0.0 | 0.1 | 0.0 | 0.2 |
| Initial Q Delay (d3), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Control Delay (d), s/veh | 0.0 | 4.6 | 0.0 | 6.4 | 0.0 | 4.7 | 0.0 | 6.6 |
| 1st-Term Q (Q1), veh/ln | 0.0 | 0.1 | 0.0 | 0.1 | 0.0 | 0.1 | 0.0 | 0.1 |
| 2nd-Term Q (Q2), veh/ln | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 3rd-Term Q (Q3), veh/ln | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \%ile Back of Q Factor (f_B\%) | 0.00 | 1.00 | 0.00 | 1.00 | 0.00 | 1.00 | 0.00 | 1.00 |
| \%ile Back of Q (50\%), veh/ln | 0.0 | 0.1 | 0.0 | 0.1 | 0.0 | 0.1 | 0.0 | 0.1 |
| \%ile Storage Ratio (RQ\%) | 0.00 | 0.00 | 0.00 | 0.01 | 0.00 | 0.01 | 0.00 | 0.01 |
| Initial Q (Qb), veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Final (Residual) Q (Qe), veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Sat Delay (ds), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Sat Q (Qs), veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Sat Cap (cs), veh/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Initial Q Clear Time (tc), h | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Middle Lane Group Data |  |  |  |  |  |  |  |  |
| Assigned Mvmt | 0 | 2 | 0 | 4 | 0 | 6 | 0 | 8 |
| Lane Assignment |  |  |  |  |  |  |  |  |
| Lanes in Grp | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Grp Vol (v), veh/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Grp Sat Flow (s), veh/h/ln | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Q Serve Time ( g _s), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Cycle Q Clear Time (g_c), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Lane Grp Cap (c), veh/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| V/C Ratio (X) | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Avail Cap (c_a), veh/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Upstream Filter (I) | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Uniform Delay (d1), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Incr Delay (d2), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Initial Q Delay (d3), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Control Delay (d), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 1st-Term Q (Q1), veh/ln | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 2nd-Term Q (Q2), veh/ln | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |

HCM 6th Signalized Intersection Capacity Analysis
3: 1st Avenue S/1st Avenue N \& Spruce Street W/Spruce Street E

| 3rd-Term Q (Q3), veh/ln | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| \%ile Back of Q Factor (f_B\%) | 0.00 | 1.00 | 0.00 | 1.00 | 0.00 | 1.00 | 0.00 | 1.00 |
| \%ile Back of Q (50\%), veh/ln | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \%ile Storage Ratio (RQ\%) | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Initial Q (Qb), veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Final (Residual) Q (Qe), veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Sat Delay (ds), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Sat Q (Qs), veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Sat Cap (cs), veh/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Initial Q Clear Time (tc), h | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |


| Right Lane Group Data |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Assigned Mvmt | 0 | 12 | 0 | 14 | 0 | 16 | 0 | 18 |
| Lane Assignment |  |  |  |  |  |  |  |  |
| Lanes in Grp | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Grp Vol (v), veh/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Grp Sat Flow (s), veh/h/ln | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Q Serve Time (g_s), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Cycle Q Clear Time (g_c), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Prot RT Sat Flow (s_R), veh/h/ln | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Prot RT Eff Green (g_R), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Prop RT Outside Lane (P_R) | 0.00 | 0.04 | 0.00 | 0.05 | 0.00 | 0.19 | 0.00 | 0.74 |
| Lane Grp Cap (c), veh/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| VIC Ratio (X) | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Avail Cap (c_a), veh/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Upstream Filter (I) | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Uniform Delay (d1), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Incr Delay (d2), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Initial Q Delay (d3), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Control Delay (d), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 1st-Term Q (Q1), veh/ln | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 2nd-Term Q (Q2), veh/ln | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 3rd-Term Q (Q3), veh/ln | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \%ile Back of Q Factor (f_B\%) | 0.00 | 1.00 | 0.00 | 1.00 | 0.00 | 1.00 | 0.00 | 1.00 |
| \%ile Back of Q (50\%), veh/ln | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \%ile Storage Ratio (RQ\%) | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Initial $Q(Q b)$, veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Final (Residual) Q (Qe), veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Sat Delay (ds), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Sat Q (Qs), veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Sat Cap (cs), veh/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Initial Q Clear Time (tc), h | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |


| Intersection Summary |  |
| :--- | :--- |
| HCM 6th Ctrl Delay | 5.4 |

HCM 6th LOS

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 1.3 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \$ |  |  | \& |  |  | \$ |  |  | \$ |  |
| Traffic Vol, veh/h |  | 104 | 4 | 12 | 92 | 8 | 6 | 1 | 11 | 2 | 1 | 1 |
| Future Vol, veh/h | 1 | 104 | 4 | 12 | 92 | 8 | 6 | 1 | 11 | 2 | 1 | 1 |
| Conflicting Peds, \#/hr | 4 | 0 | 15 | 15 | 0 | 4 | 15 | 0 | 15 | 4 | 0 | 4 |
| Sign Control F | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 89 | 89 | 89 | 89 | 89 | 89 | 89 | 89 | 89 | 89 | 89 | 89 |
| Heavy Vehicles, \% | 0 | 4 | 25 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Mvmt Flow | 1 | 117 | 4 | 13 | 103 | 9 | 7 | 1 | 12 | 2 | 1 | 1 |



## Appendix D: Future HCM Reports and Queueing Analysis




| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 5.9 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |  |
| Lane Configurations |  | ¢ |  |  | $\uparrow$ |  |  | $\uparrow$ |  |  | $\uparrow$ |  |  |
| Traffic Vol, veh/h | 0 | 27 | 1 | 20 | 14 | 9 | 1 | 7 | 44 | 16 | 10 | 1 |  |
| Future Vol, veh/h | 0 | 27 | 1 | 20 | 14 | 9 | 1 | 7 | 44 | 16 | 10 | 1 |  |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 1 | 1 | 0 | 0 |  |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |  |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |  |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |  |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |  |
| Grade, \% | - | 0 |  |  | 0 | - | - | 0 | - | - | 0 | - |  |
| Peak Hour Factor | 73 | 73 | 73 | 73 | 73 | 73 | 73 | 73 | 73 | 73 | 73 | 73 |  |
| Heavy Vehicles, \% | 0 | 9 | 0 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Mumt Flow | 0 | 37 | 1 | 27 | 19 | 12 | 1 | 10 | 60 | 22 | 14 | 1 |  |



|  | 4 | $\rightarrow$ | $\cdots$ |  |  | 4 | 4 | 4 | 7 | ( | $\dagger$ | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \& |  |  | \& |  |  | \& |  |  | \& |  |
| Traffic Volume (veh/h) | 31 | 47 | 4 | 5 | 21 | 74 | 5 | 103 | 16 | 65 | 65 | 30 |
| Future Volume (veh/h) | 31 | 47 | 4 | 5 | 21 | 74 | 5 | 103 | 16 | 65 | 65 | 30 |
| Initial Q (Qb), veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 0.99 |  | 0.97 | 0.99 |  | 0.99 | 0.99 |  | 0.96 | 0.99 |  | 0.99 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach |  | No |  |  | No |  |  | No |  |  | No |  |
| Adj Sat Flow, veh/h/ln | 1856 | 1856 | 1856 | 1811 | 1811 | 1811 | 1841 | 1841 | 1841 | 1841 | 1841 | 1841 |
| Adj Flow Rate, veh/h | 38 | 58 | 5 | 6 | 26 | 91 | 6 | 127 | 20 | 80 | 80 | 37 |
| Peak Hour Factor | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 |
| Percent Heavy Veh, \% | 3 | 3 | 3 | 6 | 6 | 6 | 4 | 4 | 4 | 4 | 4 | 4 |
| Cap, veh/h | 370 | 213 | 16 | 217 | 72 | 231 | 215 | 464 | 71 | 415 | 250 | 89 |
| Arrive On Green | 0.20 | 0.20 | 0.20 | 0.20 | 0.20 | 0.20 | 0.31 | 0.31 | 0.31 | 0.31 | 0.31 | 0.31 |
| Sat Flow, veh/h | 494 | 1075 | 82 | 49 | 360 | 1164 | 31 | 1511 | 232 | 444 | 815 | 291 |
| Grp Volume(v), veh/h | 101 | 0 | 0 | 123 | 0 | 0 | 153 | 0 | 0 | 197 | 0 | 0 |
| Grp Sat Flow(s),veh/h/ln | 1651 | 0 | 0 | 1574 | 0 | 0 | 1774 | 0 | 0 | 1550 | 0 | 0 |
| Q Serve(g_s), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.2 | 0.0 | 0.0 |
| Cycle Q Clear(g_c), s | 0.9 | 0.0 | 0.0 | 1.2 | 0.0 | 0.0 | 1.2 | 0.0 | 0.0 | 1.6 | 0.0 | 0.0 |
| Prop In Lane | 0.38 |  | 0.05 | 0.05 |  | 0.74 | 0.04 |  | 0.13 | 0.41 |  | 0.19 |
| Lane Grp Cap(c), veh/h | 600 | 0 | 0 | 520 | 0 | 0 | 750 | 0 | 0 | 754 | 0 | 0 |
| V/C Ratio(X) | 0.17 | 0.00 | 0.00 | 0.24 | 0.00 | 0.00 | 0.20 | 0.00 | 0.00 | 0.26 | 0.00 | 0.00 |
| Avail Cap(c_a), veh/h | 1833 | 0 | 0 | 1756 | 0 | 0 | 1948 | 0 | 0 | 1758 | 0 | 0 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(l) | 1.00 | 0.00 | 0.00 | 1.00 | 0.00 | 0.00 | 1.00 | 0.00 | 0.00 | 1.00 | 0.00 | 0.00 |
| Uniform Delay (d), s/veh | 6.2 | 0.0 | 0.0 | 6.3 | 0.0 | 0.0 | 4.8 | 0.0 | 0.0 | 4.9 | 0.0 | 0.0 |
| Incr Delay (d2), s/veh | 0.1 | 0.0 | 0.0 | 0.2 | 0.0 | 0.0 | 0.1 | 0.0 | 0.0 | 0.2 | 0.0 | 0.0 |
| Initial Q Delay(d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \%ile BackOfQ(50\%),veh/ln | 0.1 | 0.0 | 0.0 | 0.2 | 0.0 | 0.0 | 0.1 | 0.0 | 0.0 | 0.2 | 0.0 | 0.0 |
| Unsig. Movement Delay, s/veh |  |  |  |  |  |  |  |  |  |  |  |  |
| LnGrp Delay(d),s/veh | 6.3 | 0.0 | 0.0 | 6.6 | 0.0 | 0.0 | 4.9 | 0.0 | 0.0 | 5.1 | 0.0 | 0.0 |
| LnGrp LOS | A | A | A | A | A | A | A | A | A | A | A | A |
| Approach Vol, veh/h |  | 101 |  |  | 123 |  |  | 153 |  |  | 197 |  |
| Approach Delay, s/veh |  | 6.3 |  |  | 6.6 |  |  | 4.9 |  |  | 5.1 |  |
| Approach LOS |  | A |  |  | A |  |  | A |  |  | A |  |
| Timer - Assigned Phs |  | 2 |  | 4 |  | 6 |  | 8 |  |  |  |  |
| Phs Duration (G+Y+Rc), s |  | 10.1 |  | 8.1 |  | 10.1 |  | 8.1 |  |  |  |  |
| Change Period (Y+Rc), s |  | 4.5 |  | 4.5 |  | 4.5 |  | 4.5 |  |  |  |  |
| Max Green Setting (Gmax), s |  | 18.0 |  | 18.0 |  | 18.0 |  | 18.0 |  |  |  |  |
| Max Q Clear Time (g_c+11), s |  | 3.2 |  | 2.9 |  | 3.6 |  | 3.2 |  |  |  |  |
| Green Ext Time (p_c), s |  | 0.6 |  | 0.4 |  | 0.9 |  | 0.5 |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| HCM 6th Ctrl Delay |  |  | 5.6 |  |  |  |  |  |  |  |  |  |
| HCM 6th LOS |  |  | A |  |  |  |  |  |  |  |  |  |


|  | 4 | $\rightarrow$ | $\geqslant$ | 7 |  | 4 | 4 | 4 | P |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | ¢ |  |  | ¢ |  |  | ¢ |  |  | ¢ |  |
| Traffic Volume (veh/h) | 31 | 47 | 4 | 5 | 21 | 74 | 5 | 103 | 16 | 65 | 65 | 30 |
| Future Volume (veh/h) | 31 | 47 | 4 | 5 | 21 | 74 | 5 | 103 | 16 | 65 | 65 | 30 |
| Number | 7 | 4 | 14 | 3 | 8 | 18 | 5 | 2 | 12 | 1 | 6 | 16 |
| Initial Q, veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj (A_pbT) | 0.99 |  | 0.97 | 0.99 |  | 0.99 | 0.99 |  | 0.96 | 0.99 |  | 0.99 |
| Parking Bus Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach |  | No |  |  | No |  |  | No |  |  | No |  |


| Adj Sat Flow, veh/h/ln | 1856 | 1856 | 1856 | 1811 | 1811 | 1811 | 1841 | 1841 | 1841 | 1841 | 1841 | 1841 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Adj Flow Rate, veh/h | 38 | 58 | 5 | 6 | 26 | 91 | 6 | 127 | 20 | 80 | 80 | 37 |
| Peak Hour Factor | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 | 0.81 |
| Percent Heavy Veh, \% | 3 | 3 | 3 | 6 | 6 | 6 | 4 | 4 | 4 | 4 | 4 | 4 |
| Opposing Right Turn Influence | Yes |  |  | Yes |  |  | Yes |  |  | Yes |  |  |
| Cap, veh/h | 370 | 213 | 16 | 217 | 72 | 231 | 215 | 464 | 71 | 415 | 250 | 89 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Prop Arrive On Green | 0.20 | 0.20 | 0.20 | 0.20 | 0.20 | 0.20 | 0.31 | 0.31 | 0.31 | 0.31 | 0.31 | 0.31 |
| Unsig. Movement Delay |  |  |  |  |  |  |  |  |  |  |  |  |
| Ln Grp Delay, s/veh | 6.3 | 0.0 | 0.0 | 6.6 | 0.0 | 0.0 | 4.9 | 0.0 | 0.0 | 5.1 | 0.0 | 0.0 |
| Ln Grp LOS | A | A | A | A | A | A | A | A | A | A | A | A |
| Approach Vol, veh/h |  | 101 |  |  | 123 |  |  | 153 |  |  | 197 |  |
| Approach Delay, s/veh |  | 6.3 |  |  | 6.6 |  |  | 4.9 |  |  | 5.1 |  |
| Approach LOS |  | A |  |  | A |  |  | A |  |  | A |  |


| Timer: | 1 | 2 | 3 | 4 | 5 | 6 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |


| Left Lane Group Data |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Assigned Mvmt | 0 | 5 | 0 | 7 | 0 | 1 | 0 | 3 |
| Lane Assignment |  | $L+T+R$ |  | $L+T+R$ |  | $L+T+R$ |  | $L+T+R$ |

HCM 6th Signalized Intersection Capacity Analysis
3: 1st Avenue S/1st Avenue N \& Spruce Street W/Spruce Street E

| Lanes in Grp | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Grp Vol (v), veh/h | 0 | 153 | 0 | 101 | 0 | 197 | 0 | 123 |
| Grp Sat Flow (s), veh/h/ln | 0 | 1774 | 0 | 1651 | 0 | 1550 | 0 | 1574 |
| Q Serve Time (g_s), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.2 | 0.0 | 0.0 |
| Cycle Q Clear Time (g_c), s | 0.0 | 1.2 | 0.0 | 0.9 | 0.0 | 1.6 | 0.0 | 1.2 |
| Perm LT Sat Flow (s_l), veh/h/ln | 0 | 1281 | 0 | 1288 | 0 | 1247 | 0 | 1352 |
| Shared LT Sat Flow (s_sh), veh/h/ln | 0 | 1836 | 0 | 1817 | 0 | 1796 | 0 | 1806 |
| Perm LT Eff Green (g_p), s | 0.0 | 5.6 | 0.0 | 3.6 | 0.0 | 5.6 | 0.0 | 3.6 |
| Perm LT Serve Time (g_u), s | 0.0 | 4.0 | 0.0 | 2.4 | 0.0 | 4.4 | 0.0 | 2.8 |
| Perm LT Q Serve Time (g_ps), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.2 | 0.0 | 0.0 |
| Time to First Blk (g_f), s | 0.0 | 3.1 | 0.0 | 0.7 | 0.0 | 1.4 | 0.0 | 1.3 |
| Serve Time pre Blk (g_fs), s | 0.0 | 1.2 | 0.0 | 0.7 | 0.0 | 1.4 | 0.0 | 1.2 |
| Prop LT Inside Lane (P_L) | 0.00 | 0.04 | 0.00 | 0.38 | 0.00 | 0.41 | 0.00 | 0.05 |
| Lane Grp Cap (c), veh/h | 0 | 750 | 0 | 600 | 0 | 754 | 0 | 520 |
| V/C Ratio (X) | 0.00 | 0.20 | 0.00 | 0.17 | 0.00 | 0.26 | 0.00 | 0.24 |
| Avail Cap (c_a), veh/h | 0 | 1948 | 0 | 1833 | 0 | 1758 | 0 | 1756 |
| Upstream Filter (I) | 0.00 | 1.00 | 0.00 | 1.00 | 0.00 | 1.00 | 0.00 | 1.00 |
| Uniform Delay (d1), s/veh | 0.0 | 4.8 | 0.0 | 6.2 | 0.0 | 4.9 | 0.0 | 6.3 |
| Incr Delay (d2), s/veh | 0.0 | 0.1 | 0.0 | 0.1 | 0.0 | 0.2 | 0.0 | 0.2 |
| Initial Q Delay (d3), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Control Delay (d), s/veh | 0.0 | 4.9 | 0.0 | 6.3 | 0.0 | 5.1 | 0.0 | 6.6 |
| 1st-Term Q (Q1), veh/In | 0.0 | 0.1 | 0.0 | 0.1 | 0.0 | 0.1 | 0.0 | 0.2 |
| 2nd-Term Q (Q2), veh/ln | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 3rd-Term Q (Q3), veh/In | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \%ile Back of Q Factor (f_B\%) | 0.00 | 1.00 | 0.00 | 1.00 | 0.00 | 1.00 | 0.00 | 1.00 |
| \%ile Back of Q (50\%), veh/ln | 0.0 | 0.1 | 0.0 | 0.1 | 0.0 | 0.2 | 0.0 | 0.2 |
| \%ile Storage Ratio (RQ\%) | 0.00 | 0.01 | 0.00 | 0.01 | 0.00 | 0.01 | 0.00 | 0.01 |
| Initial $Q(Q b)$, veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Final (Residual) Q (Qe), veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Sat Delay (ds), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Sat Q (Qs), veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Sat Cap (cs), veh/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Initial Q Clear Time (tc), h | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Middle Lane Group Data |  |  |  |  |  |  |  |  |
| Assigned Mvmt | 0 | 2 | 0 | 4 | 0 | 6 | 0 | 8 |
| Lane Assignment |  |  |  |  |  |  |  |  |
| Lanes in Grp | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Grp Vol (v), veh/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Grp Sat Flow (s), veh/h/ln | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Q Serve Time (g_s), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Cycle Q Clear Time (g_c), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Lane Grp Cap (c), veh/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| V/C Ratio (X) | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Avail Cap (c_a), veh/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Upstream Filter (I) | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Uniform Delay (d1), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Incr Delay (d2), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Initial Q Delay (d3), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Control Delay (d), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 1st-Term Q (Q1), veh/ln | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 2nd-Term Q (Q2), veh/ln | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |

HCM 6th Signalized Intersection Capacity Analysis
3: 1st Avenue S/1st Avenue N \& Spruce Street W/Spruce Street E

| 3rd-Term Q (Q3), veh/ln | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| \%ile Back of Q Factor ( f B B ) | 0.00 | 1.00 | 0.00 | 1.00 | 0.00 | 1.00 | 0.00 | 1.00 |
| \%ile Back of Q (50\%), veh/ln | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \%ile Storage Ratio (RQ\%) | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Initial Q (Qb), veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Final (Residual) Q (Qe), veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Sat Delay (ds), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Sat Q (Qs), | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Sat | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Initial Q Clear , veh/h | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |


|  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\frac{\text { Right Lane Group Data }}{\text { Assigned Mvmt }}$ | 0 | 12 | 0 | 14 | 0 | 16 | 0 | 18 |
| Lane Assignment |  |  |  |  |  |  |  |  |
| Lanes in Grp | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Grp Vol (v), veh/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Grp Sat Flow (s), veh/h/ln | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Q Serve Time (g_s), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Cycle Q Clear Time (g_c), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Prot RT Sat Flow (s_R), veh/h/ln | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Prot RT Eff Green (g_R), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Prop RT Outside Lane (P_R) | 0.00 | 0.13 | 0.00 | 0.05 | 0.00 | 0.19 | 0.00 | 0.74 |
| Lane Grp Cap (c), veh/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| VIC Ratio (X) | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Avail Cap (c_a), veh/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Upstream Filter (I) | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Uniform Delay (d1), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Incr Delay (d2), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Initial Q Delay (d3), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Control Delay (d), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 1st-Term Q (Q1), veh/ln | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 2nd-Term Q (Q2), veh/ln | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 3rd-Term Q (Q3), veh/ln | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \%oile Back of Q Factor (f_B\%) | 0.00 | 1.00 | 0.00 | 1.00 | 0.00 | 1.00 | 0.00 | 1.00 |
| \%ile Back of Q (50\%), veh/ln | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \%ile Storage Ratio (RQ\%) | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Initial $Q(Q b)$, veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Final (Residual) $\mathrm{Q}(\mathrm{Qe})$, veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Sat Delay (ds), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Sat Q (Qs), veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Sat Cap (cs), veh/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Initial Q Clear Time (tc), h | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |

## Intersection Summary

HCM 6th Ctrl Delay 5.6

HCM 6th LOS



## Intersection: 1: 1st Avenue N \& School Road

| Movement | WB | NB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | LR | TR | LT |
| Maximum Queue (ft) | 68 | 36 | 87 |
| Average Queue (ft) | 32 | 4 | 26 |
| 95th Queue (ft) | 55 | 21 | 65 |
| Link Distance (ft) | 531 | 199 | 284 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 2: 2nd Avenue S/Driveway \& N Head Rd/Spruce Street W

| Movement | EB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LTR | LTR | LTR | LTR |
| Maximum Queue (ft) | 2 | 26 | 40 | 44 |
| Average Queue (ft) | 0 | 1 | 26 | 19 |
| 95th Queue (ft) | 2 | 13 | 46 | 45 |
| Link Distance (ft) | 519 | 283 | 737 | 141 |
| Upstream Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 3: 1st Avenue S/1st Avenue N \& Spruce Street W/Spruce Street E

| Movement | EB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LTR | LTR | LTR | LTR |
| Maximum Queue (ft) | 70 | 73 | 66 | 88 |
| Average Queue (ft) | 33 | 33 | 25 | 38 |
| 95th Queue (ft) | 58 | 57 | 59 | 76 |
| Link Distance (ft) | 283 | 482 | 675 | 502 |
| Upstream Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

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Intersection: 4: Elizabeth Ave SE/Elizabeth Ave NE \& Spruce Street E

| Movement | EB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LTR | LTR | LTR | LTR |
| Maximum Queue (ft) | 14 | 35 | 35 | 33 |
| Average Queue (ft) | 0 | 3 | 16 | 4 |
| 95th Queue (ft) | 5 | 19 | 41 | 21 |
| Link Distance (ft) | 232 | 231 | 235 | 240 |
| Upstream Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
|  |  |  |  |  |
| Network Summary |  |  |  |  |
| Network wide Queuing Penalty: 0 |  |  |  |  |


[^0]:    ${ }^{1}$ WSDOT Airport Facilities and Services Report, https://www.wsdot.wa.gov/aviation/planning/systemplan/conditionassessment/ReportViewer.aspx

[^1]:    ${ }^{2}$ City of Ilwaco Comprehensive Plan, page 14. Identifies level of service definitions.
    ${ }^{3}$ WSDOT Functional Classification Map, https://www.wsdot.wa.gov/data/tools/geoportal/?config=FunctionalClass

[^2]:    ${ }^{4}$ See City of Ilwaco Comprehensive Plan, page 6 for growth rate definitions.

